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Finite Element Analysis

Bar Element in 3D Space

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Lesson Outcomes

- At the end of this lesson, the student should be able to:
 - Transform bar element equations from local coordinates system to global 3D space coordinates systems
 - Analyze 3D space truss



Transformation in 3D Space

- In 3D spacy, we have 3 orthogonal axes
- Consider a vector in local coordinates system to be transformed into global space
- $u'\mathbf{i}' + v'\mathbf{j}' + w'\mathbf{k}' = u\mathbf{i} + v\mathbf{j} + w\mathbf{k}$
- Taking dot product with i^\prime
- $u' + 0 + 0 = u(\mathbf{i}'.\mathbf{i}) + v(\mathbf{i}'.\mathbf{j}) + w(\mathbf{i}'.\mathbf{k})$
- We know that

• **i**'. **i** =
$$\frac{x_2 - x_1}{L} = C_x$$
 where $C_x = \cos \theta_x$

• **i**'. **j** = $\frac{y_2 - y_1}{L} = C_y$ where $C_y = \cos \theta_y$

• **i**'. **i** =
$$\frac{z_2 - z_1}{L} = C_z$$
 where $C_z = \cos \theta_z$

• $L = \sqrt{(x_2 - x_2)^2 + (y_2 - y_2)^2 + (z_2 - z_2)^2}$



Transformation in 3D Space (Continued)

- Therefore:
- $u' = C_x u + C_y v + C_z w$
- For both ends:
- $u_1' = C_x u_1 + C_y v_1 + C_z w_1$
- $u_2' = C_x u_2 + C_y v_2 + C_z w_2$
- Matrix form

•
$$\begin{cases} u_1' \\ u_2' \end{cases} = \begin{bmatrix} C_x & C_y & C_z & 0 & 0 & 0 \\ 0 & 0 & 0 & C_x & C_y & C_z \end{bmatrix} \begin{cases} u_1 \\ v_1 \\ w_1 \\ u_2 \\ v_2 \\ w_2 \end{cases}$$



Transformation in 3D Space (Continued)

- The transformation matrix in 3D space is therefore obtained as:
- $[T] = \begin{bmatrix} C_x & C_y & C_z & 0 & 0 & 0 \\ 0 & 0 & 0 & C_x & C_y & C_z \end{bmatrix}$
- We can use this matrix to transform local deformations into global
- $\{d'\} = [T]\{d\}$
- $\{f'\} = [T]\{f\}$
- Now the basic FEA equation in local coordinates systems is given as:
- $\{f'\} = [k']\{d'\}$
- Using the transformations obtained above:
- $[T]{f} = [k'][T]{d}$
- $\{f\} = [T]^T [k'] [T] \{d\}$
- $\{f\} = [k]\{d\}$
- $[k] = [T]^T [k'] [T]$



Global Stiffness Matrix in 3D Space

• After performing the necessary matrix operations:

•
$$[k] = \frac{AE}{L} \begin{bmatrix} C_{\chi}^{2} & C_{\chi}C_{y} & C_{\chi}C_{z} & -C_{\chi}^{2} & -C_{\chi}C_{y} & -C_{\chi}C_{z} \\ C_{\chi}C_{y} & C_{y}^{2} & C_{y}C_{z} & -C_{\chi}C_{y} & -C_{y}^{2} & -C_{y}C_{z} \\ C_{\chi}C_{z} & C_{y}C_{z} & C_{z}^{2} & -C_{\chi}C_{z} & -C_{y}C_{z} & -C_{y}^{2} \\ -C_{\chi}^{2} & -C_{\chi}C_{y} & -C_{\chi}C_{z} & C_{\chi}^{2} & C_{\chi}C_{y} & C_{\chi}C_{z} \\ -C_{\chi}C_{y} & -C_{y}^{2} & -C_{y}C_{z} & C_{\chi}C_{y} & C_{y}^{2} & C_{y}C_{z} \\ -C_{\chi}C_{z} & -C_{y}C_{z} & -C_{y}^{2} & C_{\chi}C_{z} & C_{y}C_{z} \end{bmatrix}$$

- This is the general form of the stiffness matrix for a bar element orientated arbitrarily in 3D space
- Generally, this matrix is divided into 4 portions for easy assembly



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Global Stiffness Matrix in 3D Space (Continued)

•
$$[k] = \begin{bmatrix} [\lambda] & -[\lambda] \\ -[\lambda] & [\lambda] \end{bmatrix}$$

• Where

•
$$[\lambda] = \begin{bmatrix} C_x^2 & C_x C_y & C_x C_z \\ C_x C_y & C_y^2 & C_y C_z \\ C_x C_z & C_y C_z & C_z^2 \end{bmatrix}$$

- It can be observed that even this quarter of the stiffness matrix is symmetric in itself
- Therefore, one only needs to calculate 5 values that are then repeated





Author Information

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