# THEORY OF STRUCTURES <br> CHAPTER 3 : MOMENT DISTRIBUTION (FOR BEAM) PART 3 

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## Chapter 3 : Part 3 - Slope Deflection

- Aims
- Determine the end moment for beam using Moment Distribution Method
- Expected Outcomes :
- Able to do moment distribution for beams.
- References
- Mechanics of Materials, R.C. Hibbeler, 7th Edition, Prentice Hall
- Structural Analysis, Hibbeler, 7th Edition, Prentice Hall
- Structural Analysis, SI Edition by Aslam Kassimali,Cengage Learning
- Structural Analysis, Coates, Coatie and Kong
- Structural Analysis - A Classical and Matrix Approach, Jack C. McCormac and James K. Nelson, Jr., 4th Edition, John Wiley
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## MOMENT DISTRIBUTION METHOD

## SIGN CONVENTION

-Clockwise moment consider positive, whereas counterclockwise moment are negative.

## FIXED END MOMENTS (FEMs)

-Can be determined from the table.

## MEMBER STIFFNESS FACTOR



$$
\begin{aligned}
& K=\frac{4 E I}{L} \\
& \text { FAR END FIXED }
\end{aligned}
$$

Stiffness factor at A can be defined as the amount of moment $M$ required to rotate the end $A$ of the beam $=1 \mathrm{rad}$

MODIFICATIONS.....


$$
K=\frac{3 E I}{L}
$$

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## JOINT STIFFNESS FACTOR

-The total stiffness factor is a sum of the member stiffness factor at the joint.


$$
\begin{aligned}
K_{T} & =\sum K \\
& =K_{A B}+K_{A D}+K_{A C}
\end{aligned}
$$

## CARRY OVER

In MDM, we have to analyze the effects of applying imaginary moments at a specified point


The beam in Figure, when it receives a moment M at A , will develop at B moment of $\mathrm{M} / 2$.

This M/2 is called the carry over moment
If the far end B were hinged, the CO will be zero

## DISTRIBUTION FACTOR (DF)

A moment which tends to rotate without translation a joint to which several members are connected will be divided amongst the connected members in proportion to their stiffnesses.


Assumption:
Connected at A
Rotation Өis given to joint $A$ by external moment
i. The rotation of each member at A is obviously $\theta$
ii. The moments MAB, MAC, MAD (assuming up to M0 will be in ratio k1;k2:k3

## DISTRIBUTION FACTOR (DF)

$$
M_{i}=K_{i} \theta
$$

$$
M=\theta \sum K_{i}
$$

$$
D F_{i}=\frac{M_{i}}{M}=\frac{K_{i} \theta}{\theta \sum K_{i}}
$$

$$
D F=\frac{K}{\sum K}
$$

## EXAMPLE 1

A continuous beam ABC is shown in figure below. Analyse the beam to for its end moment and draw the shear force and bending moment diagram. Assume El is constant


## Solution...

i-Distribution Factor (DF)

| JOINT | MEMBER | K | $\sum K$ | DF |
| :---: | :---: | :---: | :---: | :---: |
| $A$ | $A B$ | $4 E I / 25$ | $4 E I / 25+\infty$ | 0 |
| $B$ | $B A$ | $4 E I / 25$ | $8 E I / 25$ | 0.5 |
|  | $B C$ | $4 E I / 25$ |  | 0.5 |
| $C$ | $C D$ | $4 E I / 25$ | $4 E I / 25+\infty$ | 0 |

ii-Fixed End Moment

$$
\begin{aligned}
& M_{A B}^{F}=\frac{-20(12.5)(12.5)^{2}}{25^{2}}=-62.5 \mathrm{kNm} \\
& M_{B A}^{F}=+62.5 \mathrm{kNm} \\
& M_{B C}^{F}=\frac{-(20)(15)(10)^{2}}{25^{2}}=-48 \mathrm{kNm} \\
& M_{B C}^{F}=\frac{-(20)(15)^{2}(10)}{25^{2}}=+72 \mathrm{kNm}
\end{aligned}
$$

iii-Distribution Table

| JOINT | A | B |  | C |
| :---: | :--- | ---: | :--- | ---: |
| M EM BER | AB | BA | BC | CB |
| D.F | 0 | 0.5 | 0.5 | 0 |
| FEM | -62.5 | 62.5 | -48 | 72 |
| BAL | 0 | -7.25 | -7.25 | 0 |
| C.O | -3.63 | 0 | 0 | -3.63 |
| BAL | 0 | 0 | 0 | 0 |
| END MOM ENT | -66.13 | +55.25 | -55.25 | 68.37 |



## Food of mind

Determine the internal moments at each support of the beam shown in figure below. El is constant.


## EXAMPLE 2

Determine the internal moment at each support of the beam shown in figure below. The moment of inertial of each span is indicated.


## Solution..

The moment does not get distributed in the overhanging span AB, so the distributed factor $(D F)_{A B}=0$ and $\left(D F \ni_{B} \neq 0\right.$


Solution...
i-Distribution Factor (DF)

| JOINT | MEMBER | K | $\sum K$ | DF |
| :---: | :---: | :---: | :---: | :---: |
| A | AB | - | - | - |
| B | BA | - | $4(1.5 E I) / 4$ | 0 |
|  | BC | $4(1.5 E I) / 4$ |  | 1.0 |
| C | CB | $4(1.5 E I) / 4$ | $3.1 E I$ | 0.48 |
|  | CD | $4(1.2 E I) / 3$ |  | 0.52 |
| D | DC | $4(1.2 E I) / 3$ | $4(1.2 E I) / 3+\infty$ | 0 |

## THANKS

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